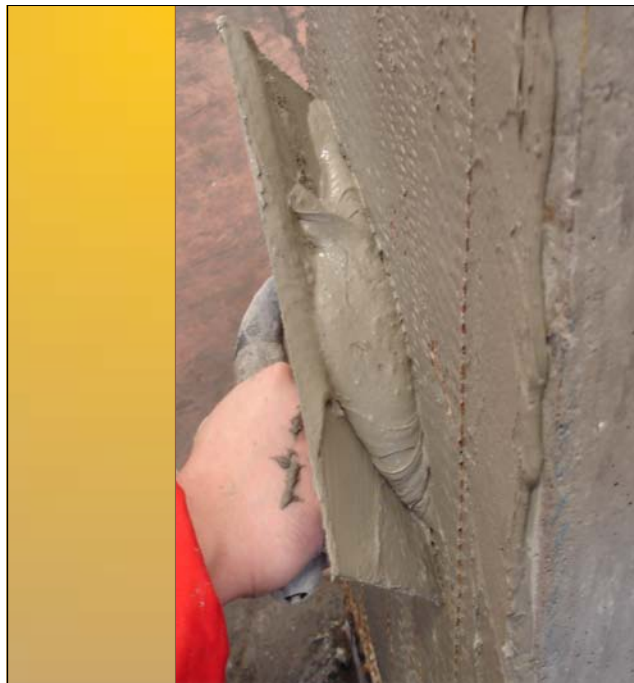




FRC M

F *iber*

R *einforced*

C *ementitious*

M *atrix*



RUREDIL

Chemical Specialities for Emerging Technologies

Direzione Tecnica

REINFORCING MATERIALS



POLIPARAFENILENBENZOBISOXAZOLO
(PBO)

CARBON(C)

+

+

CEMENTITIOUS MORTAR

EPOXY RESIN

=

=

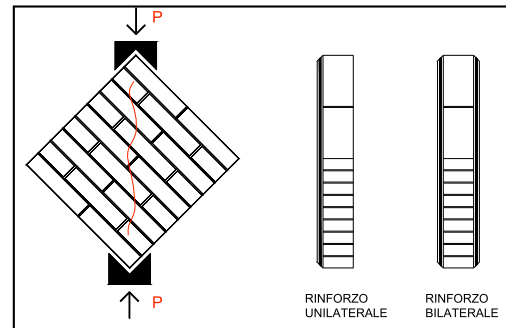
FRCM

FRP

**FRCM carbon fiber mesh
with cementitious matrix
for masonry structures strengthening**

Experimental investigation on masonry walls

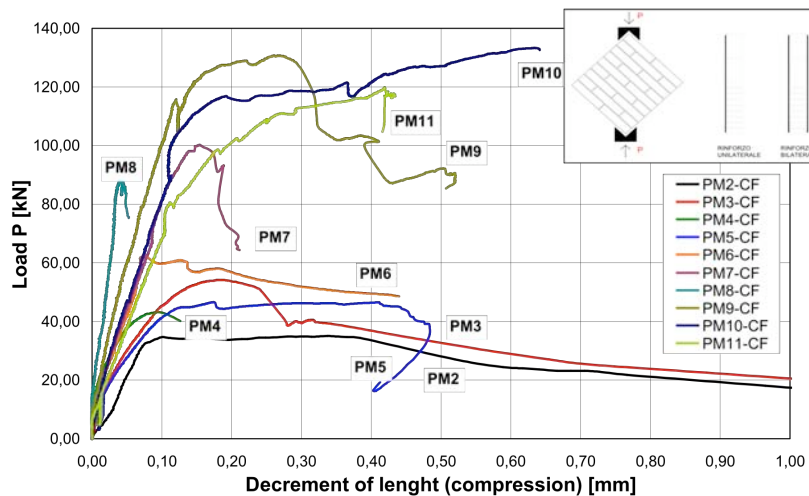
- specimen dimensions were $467,5 \times 467,5 \times 105 \text{ mm}^3$
- they were made of clay brick of $225 \times 105 \times 45 \text{ mm}^3$
- horizontal mortar joint had a mean thickness of 10 mm

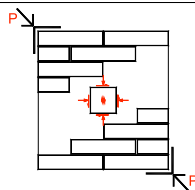


Experimental investigation on masonry walls

- ❑ control panels (PM1 and PM2)
- ❑ application of cementitious matrix on one panel side (PM3 and PM4)
- ❑ application of one FRCM layer on one panel side with carbon fibre net orientation $0^{\circ}/90^{\circ}$, as mortar joints path (PM5 and PM6)
- ❑ application of two FRCM layers on one panel side with carbon fibre orientation $0^{\circ}/90$ and $\pm 45^{\circ}$ (PM7 and PM8)
- ❑ application of two FRCM layers on both panel sides with carbon fibre orientation $0^{\circ}/90$ and $\pm 45^{\circ}$ (PM9, PM10 and PM11).

Experimental investigation on masonry walls





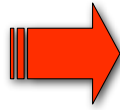
U: one side
M: only mortar

B: both sides
S: one layer

D: two layers

Definition of characteristic shear strength

Specimen	P_{max} [N]	f_{tm} [N/mm ²]	τ_k [N/mm ²]	τ_{k0}/τ_{ki}
PM2	35x10 ³	0.37	0.33	1
PM3-UM	54.2x10 ³	0.57	0.52	1.55
PM4-UM	43.2 x10 ³	0.45	0.41	1.23
PM5-US	46.4 x10 ³	0.49	0.44	1.33
PM6-US	61.9 x10 ³	0.65	0.59	1.77
PM7-UD	100.2 x10 ³	1.05	0.96	2.86
PM8-UD	88.2 x10 ³	0.93	0.84	2.52
PM9-BD	130.7 x10 ³	1.37	1.25	3.79
PM10-BD	133.3 x10 ³	1.40	1.27	3.81
PM11-BD	119.8 x10 ³	1.26	1.15	3.42



- The failure mechanism in strengthened panels was due to masonry cracking and FRCM ripping
- The strengthening system modifies the failure mechanism, increasing maximum applied force and panel stiffness. These aspects has to be considered in strengthening design.
- This technology is applicable easily by every bricklayer.

Conclusions





Mortar



Net

The system



Cutting



Application



1



2



3



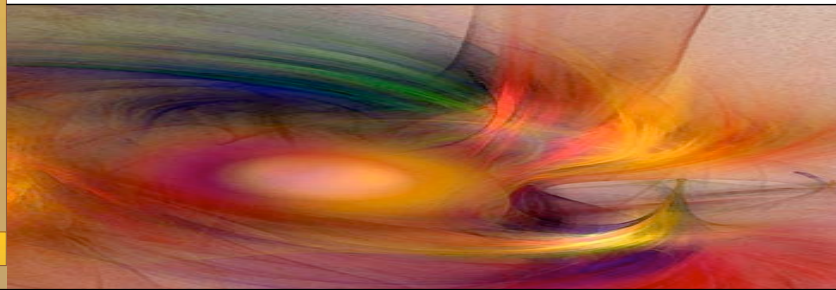
4



Ruredil X Mesh GOLD

F R C M

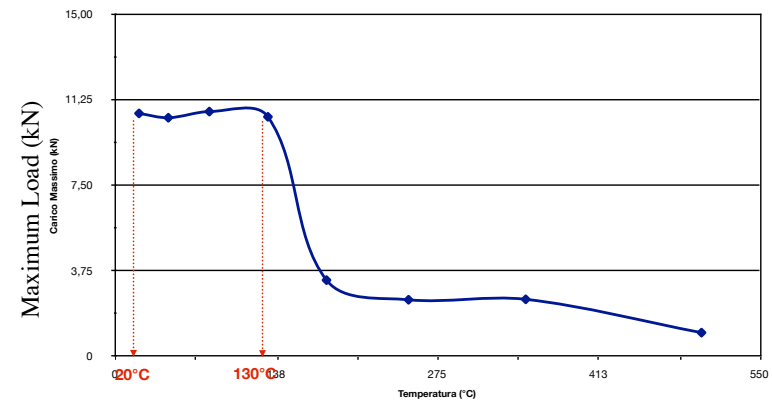
FRP vs FRCM - Temperature behaviour



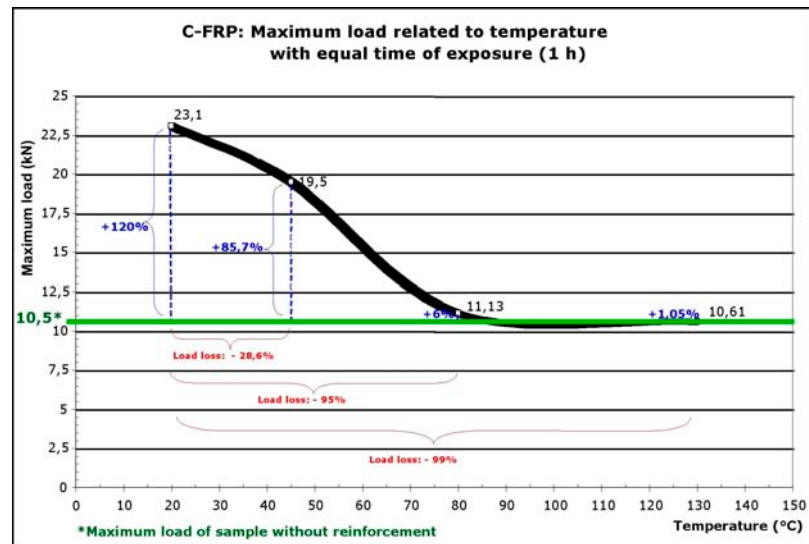
 RUREDIL

Direzione Tecnica

RESISTENZA A FLESSIONE DEL CALCESTRUZZO NON RINFORZATO IN FUNZIONE DELLA TEMPERATURA

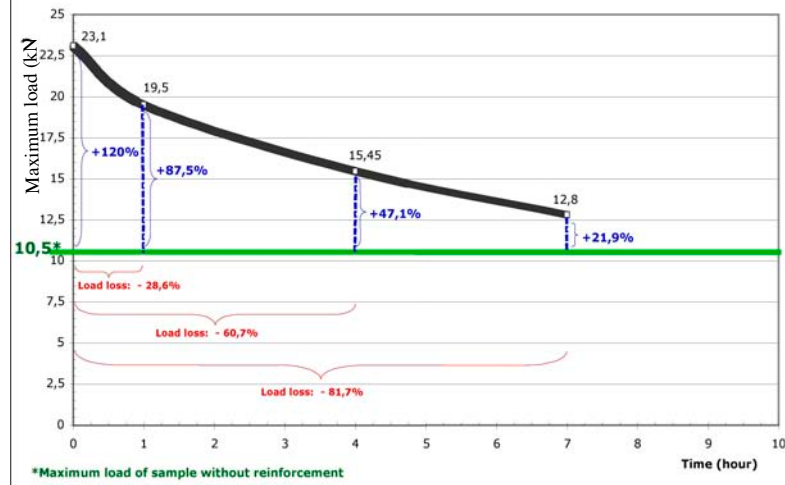


*20°C-130°C : intervallo di temperatura entro il quale la resistenza a flessione del calcestruzzo non rinforzato rimane invariata all'aumentare della temperatura





FRP: Maximum load related to time
with same temperature



Temperature and moisture test

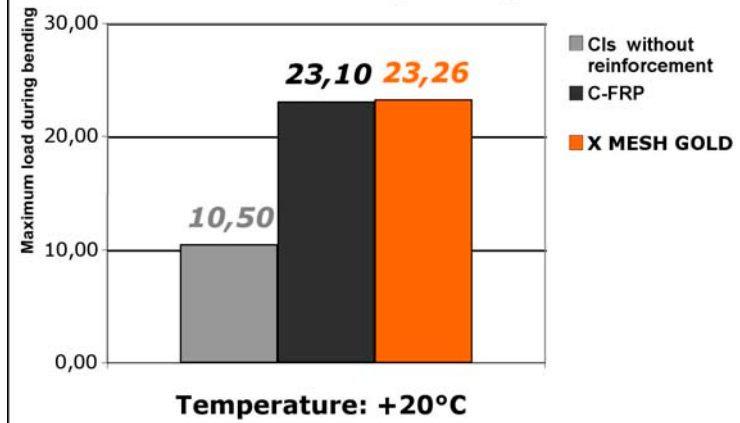
Il trattamento termico a 80°C e 100% UR (umidità relativa) è stato eseguito mediante camera di stagionatura a vapore, corredata di resistenze riscaldanti alettate, sensore d'umidità e temperatura e vaporizzatore con capacità di nebulizzazione pari a 0,5 litri/h.

Il test è stato eseguito su 120 provini in calcestruzzo delle dimensioni di 400x100x100 mm ,suddivisi in 12 serie da 10 provini cadauna:

- PBO-FRCM
- (EB) C-FRP con resina Tg= 100 °C

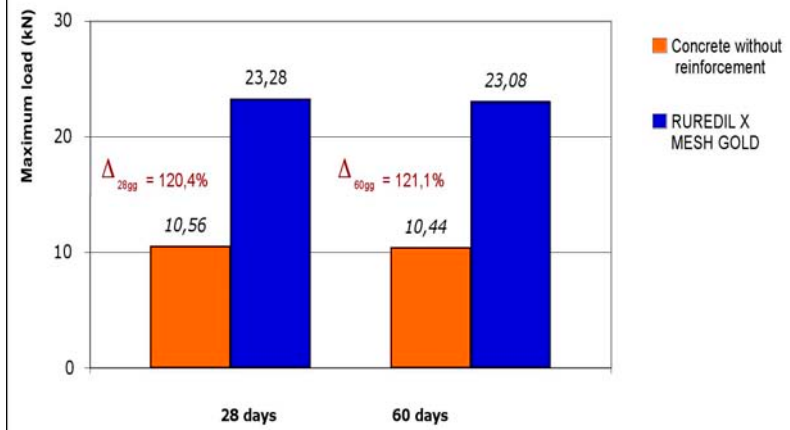


RUREDIL X MESH GOLD vs. C-FRP: Maximum load during bending





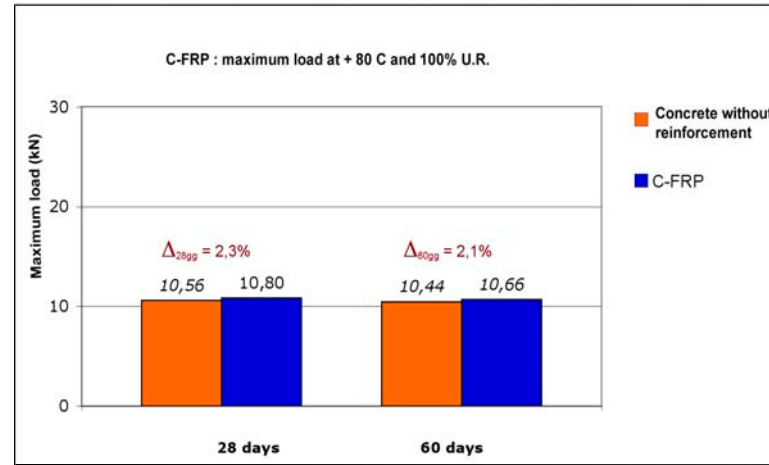
RUREDIL X MESH GOLD: maximum load at +80°C and 100% R. H.





Ruredil X Mesh GOLD

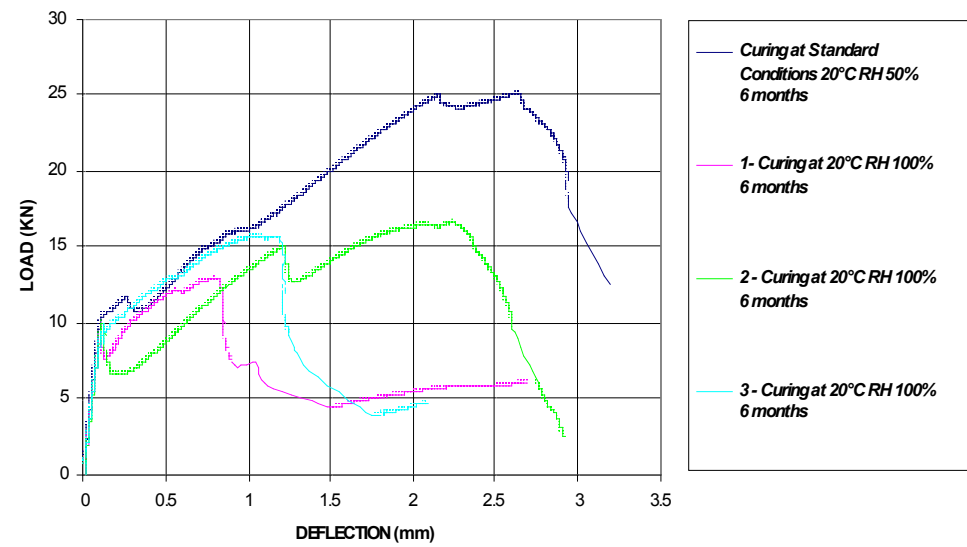
F R C M



 RUREDIL

Direzione Tecnica

LOAD-DEFLECTION



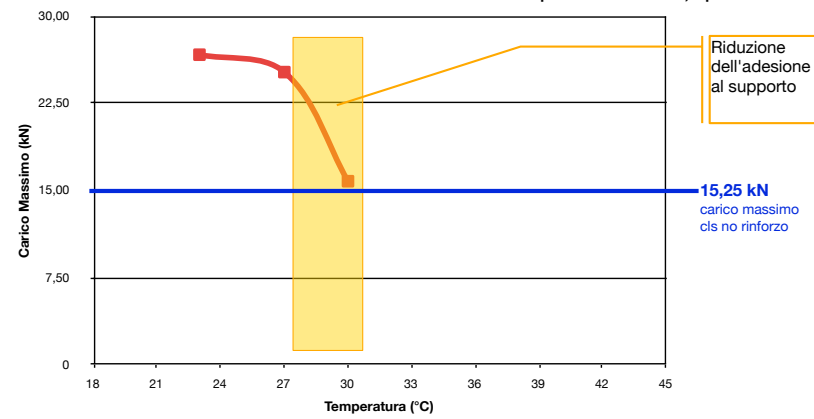
New development

C- FRP - Flexural test at constant temperature of $30^{\circ}\text{C} \pm 1$

Il test riguarda 15 provini ed è stato eseguito termostatando la macchina di prova attraverso una



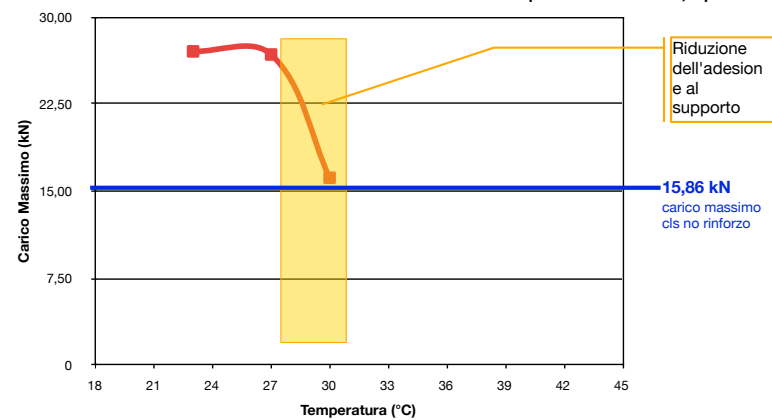
C-FRP: variazione delle caratteristiche meccaniche in funzione della temperatura di esercizio, a par



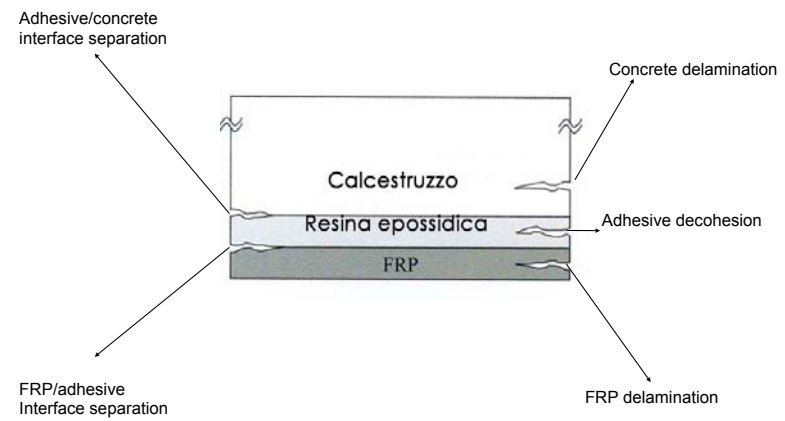
 **RUREDIL**

Direzione Tecnica

C-FRP: variazione delle caratteristiche meccaniche in funzione della temperatura di esercizio, a par



DEBONDING MODES



COHESION FRP/CONCRETE

PEELING -maximum bending moment section

design strain is the minimum value:

- characteristic rupture strain of FRP reinforcement (ε_{fu})
- maximum strain of FRP reinforcement before debonding- $\varepsilon_{f,max}$ (without environmental conditions)

$$\varepsilon_{fd} = \min \left\{ \eta_a \frac{\varepsilon_{fu}}{\gamma_f}, \eta_b \varepsilon_{f,max} \right\}$$

Peeling (maximum strain debonding-minimum value)

Material rupture

$$\gamma_f = 1.2 + 1.3$$

partial factor

$$\eta_a = 0.5 + 0.95$$

environmental conversion factor.

$$\varepsilon_{fd} =$$

maximum strain of FRP reinforcement before debonding



Compliance to UNI EN 13501-1-2007

Fire classification of FRCM and FRP :

FRCM	A1-A2	No contribution to fire / flameproofing	No i flash-over
	B-s1,d₀	No contribution to fire/ non-combustible	No i flash-over
	C	Contributioon to fire- very low	No i flash-over
	D	Contributioon to fire- low	flash-over risk
	E	Reaction to fire-very low	flash-over risk
FRP	F	No performance to reaction fire	flash-over risk

I sistemi FRP, contribuendo alla generazione e/o alla propagazione del fuoco, necessitano di una adeguata protezione con prodotti intumescenti; mentre i sistemi FRCM non sono combustibili e non contribuiscono all'incendio.

Research in progress

Edimburg University *Prof. Bisby*

Flexural beam test at temperature of 30- 200 °C/U.R. 50%

Near surface mounted (NSM) FRP
Externally-Bonded (EB) FRP
FRCM

CNR

1-Flexural test at 20 - 35 °C / U.R. 50%
2-Flexural test at U.R 60-100% / T. 23 °C

FRP
FRCM



Work in progress
2008 - 2009

Referenze - Estero

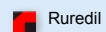
Hilandar Monastery - XII secolo

Monte Athos - Grecia

Rinforzo strutturale di circa 4000 m2



Direzione Tecnica



Ruredil

Il complesso



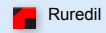
Direzione Tecnica



I dipinti

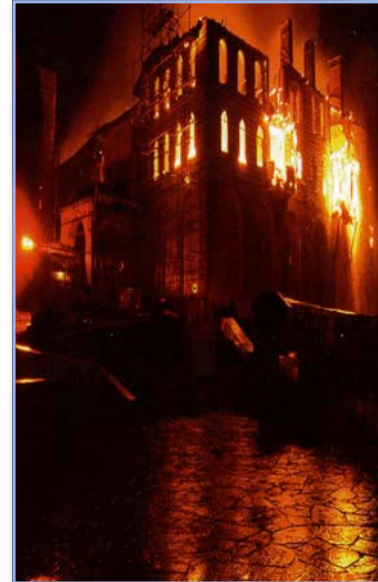
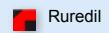
Ruredil X Mesh C10

Direzione Tecnica

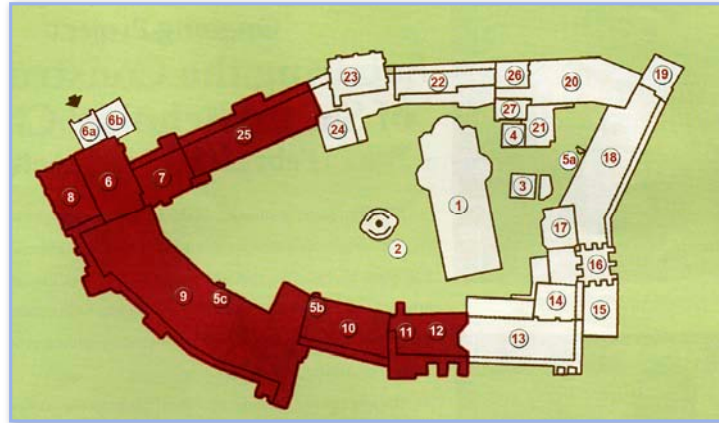


Ruredil X Mesh C10

Direzione Tecnica



Il complesso

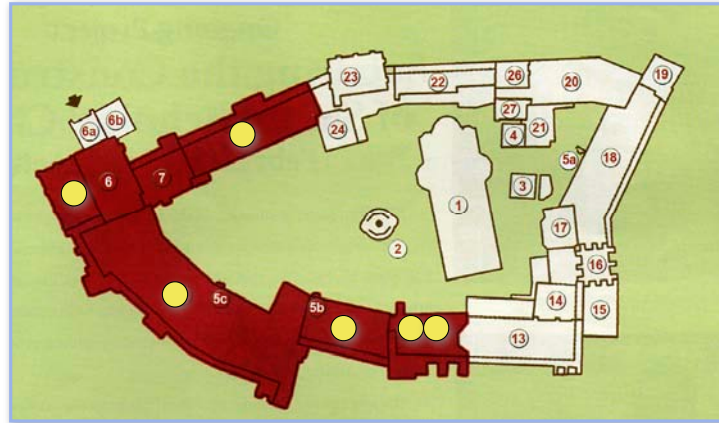


● Danneggiata dall'incendio

Direzione Tecnica

Ruredil

Il complesso



● Ricostruzione e rinforzi strutturali in corso

Direzione Tecnica

■ Ruredil

Ruredil X Mesh C10

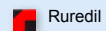
Direzione Tecnica



La ricostruzione

Ruredil X Mesh C10

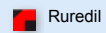
Direzione Tecnica



La ricostruzione

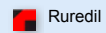
Ruredil X Mesh C10

Direzione Tecnica



Ruredil X Mesh C10

Direzione Tecnica



Il rinforzo strutturale -Ruredil X Mesh M25

Ruredil X Mesh C10

Direzione Tecnica



Il rinforzo strutturale - Ruredil X Mesh C10

Ruredil X Mesh C10

Direzione Tecnica



Ruredil X Mesh C10

Direzione Tecnica



Prime verifiche dei rinforzi strutturali eseguiti con FRCM



CAPORCIANO - Santa Maria di Centurelli

Consolidamento delle volte



S. Maria dei Cinturelli





S. Maria dei Cinturelli



S. Maria dei Cinturelli



CAPORCIANO - Santa Maria di Centurelli

Dopo il sisma del 6 Aprile 2009

2009



S. Maria dei Cinturelli



2009

S. Maria dei Cinturelli



S. Maria dei Cinturelli

2009





L'Aquila

FRCM

L'Aquila - Basilica di San Bernardino





CHIETI- Palazzo de Mayo

Consolidamento delle volte (mq 3000) - Nessuna lesione



CHIETI- Palazzo de Mayo

Palazzo De Mayo



Palazzo De Mayo



Palazzo De Mayo



Palazzo De Mayo





Moscufo - Chiesa

Consolidamento delle volte (mq 250) - Nessuna lesione -



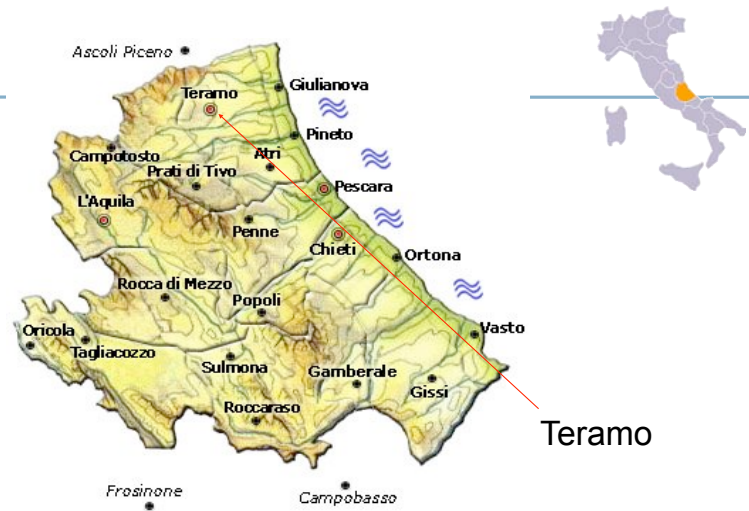


Collecorvino

Collecorvino - Convento

Consolidamento delle volte (mq 600) - Nessuna lesione -





Teramo - *Castello della Monica*

Consolidamento delle volte (mq 600) - Nessuna lesione -





Montesilvano

Montesilvano *-Ex Colonia Stella Maris*

Rinforzo Travi (mq 600) - Nessuna lesione -

